Code (UBC) as adopted by the local governing agency and any applicable local ordinances.
7. All conditions and dimensions shown on the plans shall be verified by the Contractor; any discrepancies that require clarification or revisions shall be brought to the attention of the Architect/Engineer before commencing with the work.
8. Contractor shall provide the requirements of all structural detail callouts denoted as "TYPICAL" or "TYP" at specifically noted conditions and at all like conditions throughout the project, unless otherwise noted. All details on detail sheets titled as "TYPICAL", and not directly referenced on plans, shall be incorporated at occurring locations throughout the project. Requirements of details not denoted or titled as "TYPICAL" shall be provided at the specific location shown on the plan and adjacent areas as applicable. Requirements of details denoted as "SIMILAR" or "SIM" shall be provided with differences as indicated or implied on referenced details and plans.
9. Details may be depicted diagrammatically. For example, roof pitches, floor/roof/wall thicknesses, framing members, etc., may differ in scale from actual processed conditions. Details shall be understood in context with other drawings conveying structural and architectural design intent.
10. Structural design or review of temporary shoring, additional reinforcing, bracing, formwork, scaffolding, erection methods, etc. required for proper construction of the project shall be the responsibility of the Contractor.
11. See Architectural Drawings for wall locations and dimensions, unless otherwise noted. Drawings shall not be scaled.
12. See Architectural Drawings for all flashing, waterproofing, finishes and venting requirements.
13. Refer to architectural plans for finish floor elevations, floor depressions, openings, slopes, drains, curbs, pads, embedded items, non-bearing partitions, stairs, etc. Refer to civil, mechanical and electrical plans for utilities, sleeves, pipes, ducts, equipment, etc.
14. Shop drawings are on file for field placement and are supervised by the structural drawings. It shall be the responsibility of the General Contractor to make certain that all construction is in full agreement with the latest approved contract documents.
15. Dimensions, unless otherwise shown, are to centerline of columns and beams, or to the face of concrete surfaces and rough framing.
16. All referenced publications shall be the latest edition, unless otherwise noted.
17. The contract structural drawings and specifications represent the finished structure, and, except where specifically shown, do not indicate the method or means of construction. The Contractor shall supervise and direct the work, and shall be solely responsible for all construction means, methods, procedures, techniques, safety and sequence.

S100 070218SN1 1/18/08

DESIGN CRITERIA

VERTICAL LOADS

	Dead Load (DL)	Live Load (LL)
Roof:	18 psf	16 psf
Floor (steel):	15 psf	40 psf
(concrete):	80 psf	40 psf

LATERAL LOADS
Wind Speed: 70 mph
Wind Exposure: C
Seismic Zone: 4
Seismic Base Shear: 0.23 DL (working stress)

S201L 11/7/02

STRUCTURAL OBSERVATION

The Owner shall employ MKM & Associates or another Engineer designated by MKM & Associates to perform structural observation as required by CBC Section 1709 and as defined in CBC Section 202. Items to be reviewed are:
A. Foundation reinforcing and embeds.
B. Underfloor framing (prior to insulation installation)
C. Rough framing and miscellaneous connectors.
D. Sheathed walls.
E. Floor and roof sheathing and sheathing nailing.

Contractor shall coordinate all required site reviews with his schedule and shall not cover up any work until it has been reviewed.
Contractor shall provide at least four working days notice to MKM & Associates prior to all reviews.
Structural observation is limited to the periodic visual observation of the structural system for general conformance to the approved plans and specifications at applicable construction stages and at completion of the structural system. Structural observation does not include or waive the responsibility for inspections required by the building department or special inspections required by the CBC. Job site visits by the Engineer are solely for the purpose of determining if the work of the Contractor is proceeding in accordance with the structural contract documents. This limited site observation should not be construed as exhaustive or continuous to check the quality or quantity of the work, but rather periodic in an effort to guard the Owner against defects or deficiencies in the work of the Contractor.

S106 070218SN1 1/3/08

PRODUCT SUBSTITUTIONS

Material substitutions shall be submitted to the Architect for review prior to use. Substitution reviews may require additional design costs. These additional costs shall be paid by the person or company requesting the substitution. I.C.C.
Substituted products shall have I.C.B.O. approval and shall be installed per product manufacturer's specifications. Substituted product materials, finishes, details, and installation shall be of a nature similar to originally specified product so as to not conflict with any intended structural or architectural design conditions, whether depicted or implied on plans or specifications. The substituted product shall have design values (i.e. design loads, impact resistance, etc.) which shall be equal to or greater than the originally specified product. Any and all warranties offered by the originally specified product manufacturer for the item to be substituted shall have similar warranties offered by the substituted product manufacturer.
Submit to the Architect a list of only the items to be substituted, complete with all pertinent material including but not limited to manufacturer's supplied design loads listed for the originally specified product and the proposed substitution product.

S201A2 11/7/02

CONCRETE & REINFORCEMENT

Concrete shall have a minimum compressive strength of 2500 psi at 28 days.
Minimum reinforcing steel cover requirements:
A. Cast against and exposed to earth: 3"
B. Formed surfaces: 2"
Reinforcing steel shall conform to ASTM A-615, (including supplements S1), grade 60 for #5 bars and larger and grade 40 for #4 bars and smaller. Steel shall be kept clean and free of rust scales.
Reinforcing bars shall be in lengths as long as practicable.
Reinforcing bars requiring bending in the field shall be "cold bent".
At corners and intersections bars shall return a minimum of 18". All bar splices shall have 24" laps minimum unless otherwise noted.
Reinforcing, anchor bolts and inserts shall be rigidly held in place prior to placing concrete.
Welded wire fabric shall conform with ASTM A-185, and shall be lapped 9" minimum at splices. WWF shall be placed at centerline of slab.

S202 11/8/02

A. In addition to the inspections required by Section 108 of the UBC, the Owner shall employ a Special Inspector during construction on the types of work indicated below. All special inspection shall be performed in accordance with Section 1701 of the UBC.
B. Inspections: Special inspections that are required by the building codes, local building departments, or these plans shall be performed by the firm noted in section 3.
2. SPECIAL INSPECTOR DUTIES
A. After due notice from the Architect and/or Contractor, provide qualified personnel as necessary.
B. Perform inspections as follows:
1. Perform specified reviews, inspections, sampling and testing of materials as indicated below.
2. Verify conformance of all special inspected work with the approved plans.
3. Verify that the work complies with specified standards and methods of construction.
4. Ascertain compliance of materials with requirements of the approved plans.
C. Promptly notify Architect and Contractor of observation irregularities or deficiencies within one working day. If irregularities or deficiencies are uncorrected, the Special Inspector shall notify the Architect and the governing agency.
D. Promptly submit written report of each test and inspection with a copy each to the Architect, Engineer, Owner, Contractor, Governing Agency and other designated persons within three working days. Each report shall include:
1. Date issued.
2. Project title and number.
3. Testing laboratory name, address and telephone number.
4. Name and signature of laboratory test or inspection.
5. Date and time of sampling, test or inspection.
6. Type of inspection or test.
7. Location of sample or test in the project.
8. Test results. Report shall indicate compliance or noncompliance with approved details and plans.
E. In addition to the above required reports, the Special Inspector shall submit a final signed report stating whether the work requiring special inspection was, to the best of his knowledge, in conformance with the approved plans and the applicable provisions of the Uniform Building Code. UBC 1704
3. SCOPE OF WORK (By: Special Inspector Company/Firm)
A. Grading, Excavation and Fill
By: Earth Science Consultants
During earthwork excavations, grading and filling operations.
B. Foundation Excavations
By: Earth Science Consultants
During excavation of pier holes and/or footings.
C. Reinforcing Steel
By: Earth Science Consultants
Inspection of all reinforcing steel prior to closing of forms and prior to concrete delivery.
EXCEPTION: Slab on grade.
D. Concrete
By: Earth Science Consultants
During the taking of compression test specimens, slump test and placing of all reinforced concrete and pneumatically placed concrete.
EXCEPTION: Slab on grade.
E. Welding
By: Forsythe Engineering Consultants
All structural welding shall have special continuous inspection.
* EXCEPTION: Single pass fillet welds may have periodic inspection unless otherwise noted. Prior separate approval of this exception by the Building Official is required.
F. Helical Pier Installation
By: Earth Science Consultants
During installation and testing of Helical Piers as described in the soil report.

S201J 11/7/02

FOUNDATIONS/SITE WORK

1. All site work, drainage systems, foundations and other soil considerations shall conform to the recommendations of the soil report and any subsequent recommendations made by the soil engineer of record.

SOIL ENGINEER: EARTH SCIENCE CONSULTANTS
TITLE: 6333 JERILYN STREET, RICHMOND, CALIFORNIA
JOB NO: 066119 DATE: 2-15-06

A. Soil Engineer shall review and approve site plans, foundation plans, and foundation details for conformance with design intent of soil report prior to all site work.
B. See soil report and SPECIAL INSPECTION section for all grading, fill placement and building pad preparation requirements.
C. Fill placement shall be observed by the Soil Engineer. The Soil Engineer shall prepare compaction test results of the compacted soil in order to provide a professional opinion as to conformance of the grading with the recommendations of the soil report. D. Soil Engineer shall review and approve all footing/pier depths before placement of reinforcement and forms. See SPECIAL INSPECTION section.

S201B 11/7/02

PIER AND GRADE BEAM FOUNDATIONS

1. See FOUNDATIONS/SITE WORK and CONCRETE & REINFORCEMENT sections for additional requirements.
2. All drilled piers and grade beams shall have Special Inspection. See SPECIAL INSPECTION section on this sheet.
3. Top of grade beams shall be stepped per detail on sheet S02 unless otherwise noted.
4. Grade beam widths shall be strictly maintained from step of forms to pad grade. All "slough" shall be removed before concrete sets.
5. Holes and notches are not allowed in grade beam unless shown on structural plans or approved by Engineer.
6. All slabs shall be completely separated from piers and grade beams with felt or mastic.
7. A 2" maximum diameter pipe sleeve may penetrate the grade beam vertically. Double the reinforcing bars (top and bottom) specified in detail 1/S203. Extend these additional bars 36" each side of pipe sleeve. Pipe diameter up to 3" may be used if grade beam width is 8" or increased to 8" between adjacent piers.

S003 11/7/02

A. All carpentry shall conform to the conventional construction provisions of the UBC, Section 2320 unless detailed otherwise.
B. All framing lumber shall be Douglas Fir and meet or exceed the following commercial grades, U.O.N.: Grading shall be per West Coast Lumber Inspection Bureau (WCLIB) standards or Western Wood Products Association (WWPA) standards.
* Studs less than 10" plates and blocking: Standard or better
* Joists, rafters, ledgers:
and 4x beams and posts (F.O.H.C.): No. 2
* Beams and posts 6x and larger (F.O.H.C.): No. 1
C. Lumber shall have a maximum moisture content of 19% at time of installation. Care shall be taken to allow for the effects of shrinkage. If necessary to avoid sagging, joists, rafters and beams shall be braced midspan until lumber has reached a stable moisture content.
D. Pressure Treated Lumber
1. All pressure treated lumber shall be Douglas Fir with grade per plan. (P.T.D.F.)
2. Treatment shall be ACZA, CCA or ACA and shall conform to AWPA standard C2 with the following retentions:
a. All wood in contact with foundation concrete "above ground" 0.25 retention
b. All wood embedded in concrete or in contact with earth "ground contact" 0.40 retention
3. All pressure treated lumber shall be clean, dry and free from surface residue.
4. Field treat cuts, notches, boring, etc. and handle treated lumber in accordance with AWPA Standard M4.
5. All pressure treated lumber shall carry the quality mark of an independent inspection agency qualified by ALSC.
E. All structural sheathing panels shall be identified with the appropriate trademark of the American Plywood Association, and shall meet the requirements of the latest edition of U.S. Product Standard PS-1 or NER108. All panels which have any edge or surface permanently exposed to weather shall be classified Exterior. Panel thickness, and Span Rating shall be at least equal to that shown on the drawings. Application shall be in accordance with recommendations of the American Plywood Association.
F. Glulam Beams
1. Glulams shall be DF/DF combination 24F-V4 for simple span conditions and 24F-V8 for continuous and cantilever beams with a 3500" radius camber, unless otherwise noted. Each structural glue laminated beam shall be stamped by a grade mark or have a certificate of inspection issued by an approved agency. Furnish certification of inspection to the Building Department. Glulam beams shall conform to ANSI/AITC 190.1 and ASTM D3737.
2. All exterior glulam beams exposed to weather shall be Alaska Yellow Cedar. All heartwood shall be used. Alaska Yellow Cedar glulams shall be combination 20F-V12 for simple span conditions and 20F-V12 with tension lams balanced top and bottom for continuous and cantilever applications unless otherwise noted.
G. Manufactured "I" Joists
1. Wood "I" joists shall be as manufactured by True Joist or approved equivalent. Wood "I" joists shall be installed and braced prior to sheathing according to the manufacturer's recommendations.
2. Unless otherwise noted on plan, joist hangers shall be "ITT", "IUT" or "IUS" type per Simpson Co., San Leandro, CA. Fill all holes with Simpson "N10" nails (10d x 1 1/2").
3. Holes in "I" joist webs shall not be permitted unless their size and placement conform to manufacturer's specifications.
4. Do not cut or notch top or bottom flanges of "I" joists.
5. Wood "I" joist shall be protected from moisture according to the manufacturer's recommendations.
H. Structural Composite Lumber (SCL)
1. SCL shall be Laminated Veneer or Parallel Strand lumber.
a. 1 3/4" thick SCL members shall have the following minimum properties:
E = 1.7 x 10^6 psi
Fb = 2600 psi
Fv = 285 psi
Fc (perp) = 700 psi
Fc (par) = 2350 psi
b. 3 1/2" to 7" thick SCL members shall have the following minimum properties:
E = 2.0 x 10^6 psi
Fb = 2600 psi
Fv = 285 psi
Fc (perp) = 700 psi
Fc (par) = 2750 psi
2. Multiple piece SCL beams shall not be substituted for full size SCL beams unless specifically noted on drawings.
3. Nail built-up 1-3/4" thick SCL beams with three rows of 16d nails at 12"o.c. staggered, typical unless otherwise noted.
I. FASTENERS/CONNECTORS
A. All nailing not specifically called out on plans shall be per Table 23-11-B-1 of the Uniform Building Code and table below:

CONNECTION	NAILING
Joist to sill or girder, toenail	3-8d
Bridging to joist or stud, toenail each end	2-8d
2" subfloor to joist or girder, blind and face nail	2-16d
Sole plate to joist or blocking, typical face nail	16d @ 16"o.c.
Sole plate to joist or blocking, at braced wall panels	3-16d per 16"o.c.
Top plate to stud, end nail	2-16d
Stud to sole plate	4-8d toenail or 2-16d end nail
Double studs, face nail	16d at 24"o.c.
Doubled top plates, typical face nail	16d at 16"o.c.
Doubled top plates, lap splice	8-16d
Top plates, laps and intersections, face nail	2-16d
Rim joist to top plate, toenail	8d @ 6"o.c.
Blocking between joists or rafters to top plate, toenail	3-8d
Ceiling joists to plate, toenail	3-8d
Continuous header to stud, toenail	4-8d
Ceiling joists, laps over partitions, face nail	3-16d
Ceiling joists to parallel rafters, face nail	3-16d
Rafter to plate, toenail	3-8d
Built-up corner studs	16d at 24"o.c.
Built-up beams	16d at 24"o.c. at top and bottom staggered

B. Nails shall be as indicated below U.O.N on plans.
1. Roof and floor sheathing: Common nails.
2. Sheerwall sheathing: Common or hot dipped or tumbled galvanized (electroplated box nails are not acceptable).
3. Framing: Common box, or coated sinker nails U.O.N. Nails shall be as specified.
4. Metal connectors: By Manufacturer, unless otherwise noted.
C. Where exposed to weather, use galvanized nails.
D. Where wood tends to split, replace member and predrill holes.
E. Machine bolts and anchor bolts shall conform to ASTM A-307. Threaded round stock shall conform to ASTM A36. Provide standard washers conforming to ANSI B18.22.1 under head and nut where bearing is against wood U.O.N. Bolt holes in wood shall be 1/16" larger than bolt sizes, U.O.N. Nuts shall be tightened when placed and retightened before closing in.
F. Joist hangers, metal connectors and other miscellaneous timber connectors shall be per Simpson Co., San Leandro, California. Nail or bolt at all pre-drilled holes, per manufacturers instructions, unless otherwise noted. All straps shall be centered on splice unless otherwise noted.
G. Expansion bolts shall be ICBO approved. Expansion bolts shall be installed per the manufacturer's recommendations unless otherwise noted. See plans for restrictions. Acceptable anchors:
Kwik Bolt-II by Hilti (ICBO #ER-4627)
Trubolt Wedge by ITW Ramset (ICBO #ER-1372)
Wedge-All by Simpson (ICBO #ER-3631)
Wedge-Bolt by Powers (ICBO #ER-5788).

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3. INSTALLATION
A. Solid block under all beams and posts to provide continuous support to foundation or post below.
B. All beams, joists and rafters shall be supported laterally at each end and at interior supports by solid blocking or similar framing to prevent rotation of member.
C. Do not notch beams, joists, rafters and studs unless otherwise noted, or approved by Engineer. See detail 3/S02.
D. All openings shall be between framing members, unless otherwise noted on drawings.
E. California framing shall be 2x6 at 24"o.c. rafters and 2x8 ridge and hips, unless otherwise noted. Brace California framing to typical framing below at 48"o.c. maximum. Typical roof framing and structural sheathing shall be continuous below California framed areas. Provide solid block or continuous flat 2x member under ends of all rafters.
F. Stud walls shall be framed with 2x4 studs at 16"o.c., unless otherwise noted. Frame all stud walls full height, continuous to bottom of floor joists or rafters. All exterior stud walls greater than 10 feet high shall have 2x6 studs at 16"o.c. unless otherwise noted.
G. Where cripple walls are framed with studs less than 14" high, provide solid blocking.
H. Care shall be taken to allow for the effects of lumber shrinkage. If necessary to avoid sagging, joists, rafters and beams shall be braced midspan until lumber has reached a stable moisture content.
J. Structural sheathing nails shall be driven so that their head is flush with surface of the sheathing.

S201H 11/20/02

MANUFACTURED ROOF TRUSSES

1. For truss shapes, dimensions, etc. see Architectural drawings.
2. All trusses shall be designed and manufactured in conformance with the Truss Plate Institute's (TPI) National Design Standard ANSI/TPI 1-1995 and Appendix "A" of the commentary.
3. Contractor shall submit stamped manufactured truss design drawings (signed by a Structural or Civil Engineer licensed in the State where the project is located) to the Architect and Engineer-of-Record along with the truss placement plan. These truss documents shall be forwarded to the Building Official with a notation indicating that they have been reviewed by the Engineer-of-Record and have been found to be in general compliance with the design intent of the working drawings. Trusses shall not be fabricated prior to approval by the Building Official.
4. Manufactured roof truss design criteria:
A. Wind load: 70 mph Exposure C
Truss Designer shall design gable end vertical members to span a minimum of 5'-0" with wind loads normal to exterior face.
B. Vertical Loads
Top Chord: 15 psf Dead Load
14 psf Live Load
Bottom Chord: 5 psf Dead Load
16 psf Live Load*
* Bottom chord live load does not act simultaneously with other imposed live loads. Superimposed loads from jack trusses, secondary framing, architectural finishes, mechanical units, electrical fixtures, plumbing, etc. shall be included in design of supporting trusses.
C. Maximum allowable deflections:
Dead Load (vertical) = 0.5"
Dead Load (horizontal) = 0.5"
Live Load (vertical) = Span/360
Total Load (vertical) = 1.0"
Truss Designer shall include deflection calculations with the design drawing submittal. Truss Designer shall camber all trusses for anticipated dead load deflection.
5. Truss Designer shall specify all trusses to truss connections.
6. Truss Bracing:
A. Truss Designer is responsible for the design and specification of all truss erection bracing and temporary bracing.
B. Truss Designer shall clearly indicate requirement for all chord and web bracing on shop drawings. See detail on sheet S02 for chord and web bracing details. Top and bottom chords of trusses require bracing when not connected directly to roof or ceiling sheathing.
C. Contractor will verify requirement for and install all connectors, erector bracing, temporary bracing and permanent bracing as specified by Truss Designer or as noted on approved plans.
7. Truss Designations:
DTH (XXXH) = Drag truss. Design truss to transfer the lateral force noted from top chord to bottom chord. Nail structural sheathing to top chord with 8d at 6"o.c.
GT = Girder truss. Design truss to carry tributary DL and LL forces.
GT/DTH (XXXH) = Girder truss/drag truss. Design truss for both DT and G loads. Nail structural sheathing to top chord with 8d at 6"o.c.
* All multiple ply drag trusses shall be internailed with 16d at 6"o.c. staggered along top & bottom chords
8. Provide multiple stud and/or solid blocking under all multiple trusses.
9. Where trusses are spaced at 30"o.c. as required to provide mechanical access, provide 2x4 ladder blocking at 24"o.c. at top and bottom chords and blocking at unsupported roof sheathing edges. Provide 3-8d toe nails or 2-16d end nails at each end of each block.
10. Truss manufacturer shall legibly mark each truss with the location of any interior bearing points as well as the following information located within 2 feet of the center of the span on the face of the bottom chord:
A. Identify of the company manufacturing the truss.
B. The design load.
C. The spacing of trusses

S011 11/7/02

STRUCTURAL INSULATED PANELS

1. Structural Insulated Panels (S.I.P) shall be manufactured by A.F.M. CORPORATION in accordance with ICC ES ESR-2233.

NOTE: WARRANTY OF EXISTING CONSTRUCTION IS NOT INCLUDED IN SCOPE OF WORK

CITY OF RICHMOND
CHANGE APPROVED
Change:
REQUIRED Structural Observation and Special Inspection per 2013 CBC
Reviewed by: Date:

1. MATERIAL
A. 10" Floor "I" joists shall have the following minimum properties; 1 1/4" min width stiffener flanges.
1. 6# gauge min thickness
B. Framing members and accessories shall conform to the following:
1. 16 gauge and heavier, ASTM A446, Fy = 50 k.s.i. min.
2. 18 gauge and lighter, ASTM A446, Fy = 33 k.s.i. min.
C. All metal framing components shall have a galvanized finish conforming to ASTM A525, G90 coating.
D. Structural properties shall conform to AISI, "Specification for the Design of Cold Formed Steel Structural Members".
2. CONNECTIONS
A. Connection of framing components shall be welded or screwed as recommended by the manufacturer or as indicated on the drawings.
B. Welded connections shall be made using arc welding or resistance welding methods. All welding shall be performed in accordance with the latest recommendations of the AWS D1.3 and AWS D1.3. Protection of the weld area after welding shall be accomplished using a zinc-rich galvanizing repair paint.
C. Screwed connections shall be made using self-drilling, self-tapping screws. Screw type and installation shall be I.C.B.O. approved. All screws shall be zinc or cadmium plated. See details for typical screws.
3. INSTALLATION
A. For each type of metal framing indicated on the drawings, provide and install bridging, stiffener tracks, blocking, lintels, clip angles, strapping reinforcements, fasteners, etc. as recommended by the manufacturer and as needed to provide a complete metal framing system.
B. All components shall be properly sized to fit integrally with all metal components. All components shall be of size, shape and profile shown or implied on the drawings.
C. Floor and Ceiling Joists
1. Set floor or ceiling joists parallel and level, with end bearing, lateral bracing, and bridging in accordance with manufacturer's recommendations and plans.

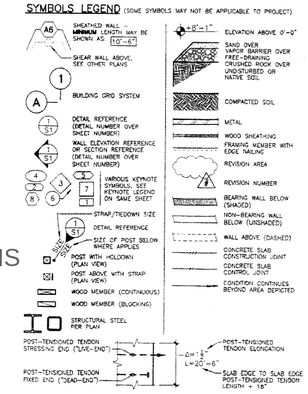
S255 12/98

STEEL

1. MATERIALS
A. Steel wide flange W-shapes shall conform to ASTM A992.
B. Steel M, S and HP shapes, structural plates, bars, angles and channels shall conform to ASTM A36 or ASTM A572, GR50.
C. Steel tubes shall conform to ASTM A500, Grade B.
D. Steel pipes shall conform to ASTM A53, Grade B.
E. A307 bolts shall be used unless otherwise noted and where specifically indicated on the drawings. A307 bolts shall conform to ASTM A307 Grade A, "Standard Specification for Carbon Steel Externally Threaded Standard Fasteners".
F. Threaded rod shall conform to ASTM F1554-1 Grade 36, unless otherwise noted.
G. Weld metal shall match base metal per AWS requirements. Electrodes for welding shall be as specified below:
1. Electrodes for structural steel:
a. SMAW: E70XX low hydrogen.
b. FCAW: E7XT-X (except -2, -3, -10, -GS) (AWS A5.20).
2. Electrodes for sheet metal less than 1/8" thick:
Electrodes and welding shall comply with AWS D1.3 latest edition.
H. All steel exposed to wet conditions shall be galvanized or primed and painted with one of the following:
1. ZINC Phosphate primer and acrylic or enamel paint.
2. Epoxy primer and paint.
I. All paint applied to steel shall be compatible with primer used.
J. All interior exposed steel shall be prime painted unless otherwise noted.
K. Steel pipes for helical piers shall conform to API J-55.
2. INSTALLATION
A. All work shall be in conformance with the AISC, "Specification for the Design, Fabrication and Erection of Structural Steel for Buildings", latest edition.
B. A307 bolts shall be tightened with an impact wrench to a snug tight condition. The snug-tight condition is defined as the tightness attained by a few impacts of an impact wrench or the full effort of a man using an ordinary spud wrench.
C. All structural steel and reinforcing steel welding shall be done in accordance with AWS latest edition and shall have special inspection per "TESTING LABORATORY SERVICES" section and CBC Section 1701.
D. All shop and field welders shall be certified according to AWS procedures for the welding process and welding position used.

S201E 2/12/03

SYMBOLS LEGEND (SOME SYMBOLS MAY NOT BE APPLICABLE TO PROJECT)



STRUCTURAL SHEET INDEX

SHEET	DESCRIPTION
SN1	STRUCTURAL NOTES
S1	FOUNDATION PLAN
SD1	STRUCTURAL DETAILS
SD2	STRUCTURAL DETAILS
SD3	STRUCTURAL DETAILS
SD4	STRUCTURAL DETAILS

JOB-SITE SET

AS BUILT DRAWING NOTES & SPECIFICATIONS

RESIDENCE FOR MR. CARL ADAMS

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REGISTERED PROFESSIONAL ENGINEER JOSEPH OAKLEY JR. No. 2000 Exp. 6-30-10 STRUCTURAL STATE OF CALIFORNIA
5/20/2008

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Drawing scales as indicated are for reference only and are not intended to accurately depict actual or designed conditions. Written dimensions shall govern.

Adams Residence
6333 Jerilyn Street
Richmond, California, 94806

REGISTERED PROFESSIONAL ENGINEER JOSEPH OAKLEY JR. No. 2000 Exp. 09/30/15 STRUCTURAL STATE OF CALIFORNIA

No.	Description	Date

3-20-08 JWC MJ
BUILDING PERMIT APPLICATION

ISSUE INFORMATION

Designer: John
PD: CP LD: BAS
Job: 070218
File: 070218SN1
Plot Date: 5-20-08
Sheet Title: STRUCTURAL NOTES
Sheet SN1

OWNER:
MR. CARL ADAMS
6333 JERILYNN AVENUE
RICHMOND, CA 94806

GENERAL NOTES AS BUILT CONSTRUCTION

Project number	2013-63
Date	1/17/2014
Drawn by	STAFF
Checked by	J.O.

S9

Scale AS NOTED